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**MAE4700/5700**

# **Finite Element Analysis for Mechanical and Aerospace Design**

**Cornell University, Fall 2009**

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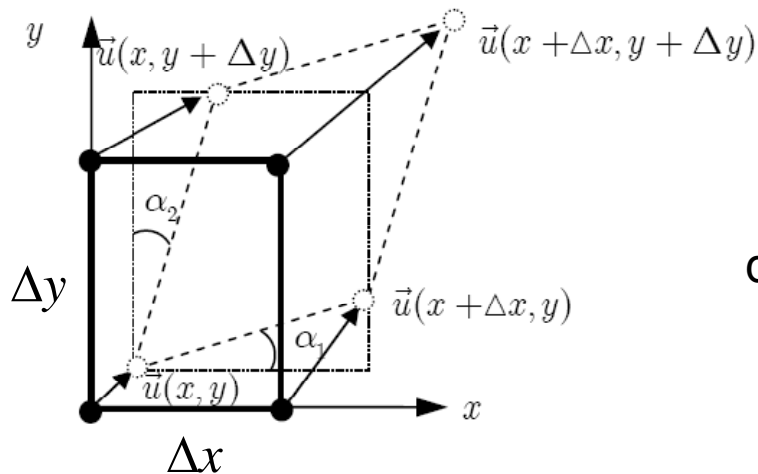
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# Kinematics in two-dimensions

- We start with the displacement vector at a given point in 2D. It is a vector with  $x$ - and the  $y$ -components. We denote it using both matrix and vector notation:

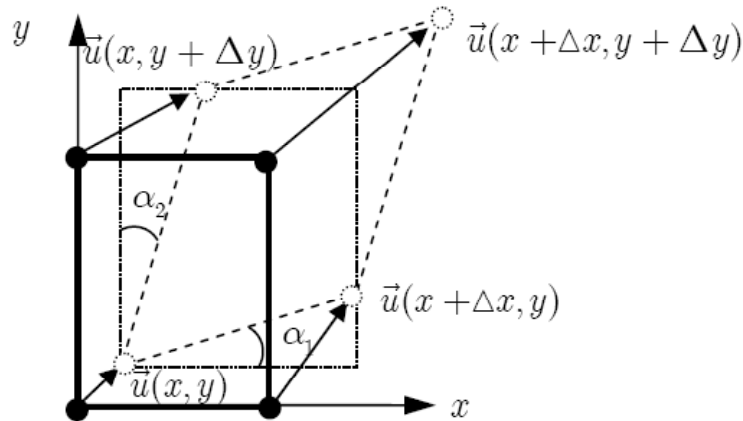
$$u = \begin{bmatrix} u_x \\ u_y \end{bmatrix} \quad \vec{u} = u_x \vec{i} + u_y \vec{j}$$

- Let us consider a square  $\Delta x \Delta y$  (control volume) on the plane before and after deformation as shown:



Can you use this figure to define the strain components?

# Extensional strain components

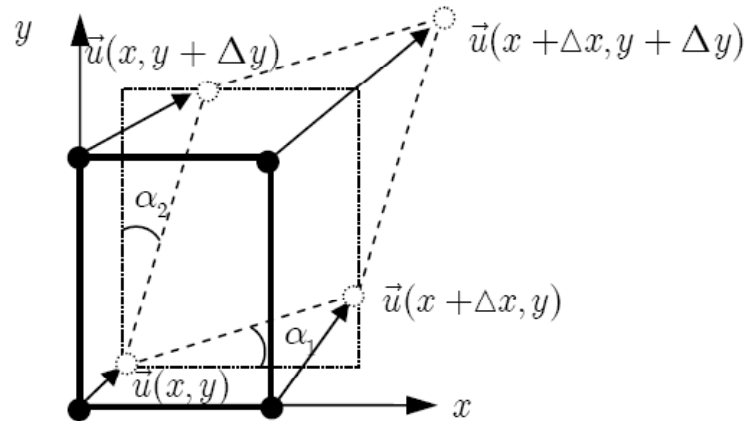


- The extensional strains  $\epsilon_{xx}$  and  $\epsilon_{yy}$  are defined as:

$$\epsilon_{xx} = \lim_{\Delta x \rightarrow 0} \frac{u_x(x + \Delta x, y) - u_x(x, y)}{\Delta x} \equiv \frac{\partial u_x}{\partial x}$$
$$\epsilon_{yy} = \lim_{\Delta y \rightarrow 0} \frac{u_y(x, y + \Delta y) - u_y(x, y)}{\Delta y} \equiv \frac{\partial u_y}{\partial y}$$

- $\epsilon_{xx}$  and  $\epsilon_{yy}$  represent the change in the lengths of the infinitesimal line segments in the  $x$  and  $y$  directions,  $\Delta x$  and  $\Delta y$ , respectively, divided by the original lengths of the line segments.

# Shear strain components



- The shear strain,  $\gamma_{xy}$ , measures the change in angle  $\alpha_1 + \alpha_2$  between unit vectors in the x and y directions (in radians)

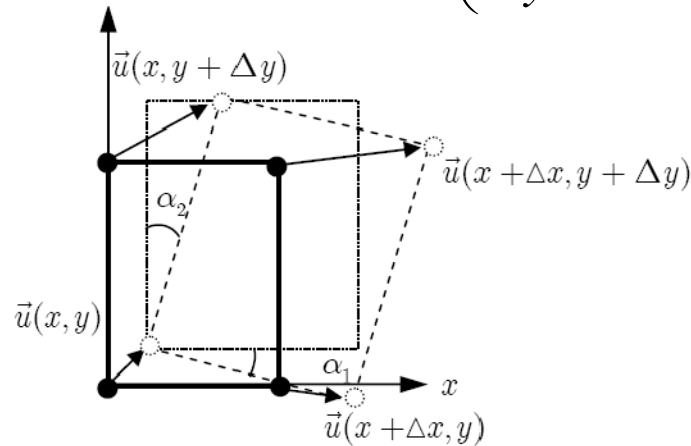
$$\gamma_{xy} = \lim_{\Delta x \rightarrow 0} \frac{u_y(x + \Delta x, y) - u_y(x, y)}{\Delta x} + \lim_{\Delta y \rightarrow 0} \frac{u_x(x, y + \Delta y) - u_x(x, y)}{\Delta y} \equiv \frac{\partial u_y}{\partial x} + \frac{\partial u_x}{\partial y}$$

- We often use the **engineering shear strain**  $\gamma_{xy}$  introduced above, and the **tensor shear strain component**  $\varepsilon_{xy} = \frac{1}{2} \gamma_{xy}$ .

# Rotation

- In addition to axial elongations, the control volume also undergoes rotation. The rotation in 2D, denoted by  $\omega_{xy}$ , is computed by

$$\omega_{xy} \equiv \frac{1}{2}(\alpha_2 - \alpha_1) = \frac{1}{2} \left( \frac{\partial u_x}{\partial y} - \frac{\partial u_y}{\partial x} \right)$$



- If  $\alpha_1 = \alpha_2$ , the rotation is zero. For small deformations, the rotation  $\omega_{xy}$  is small and does not affect the stress calculation.

# Strain matrix

- For our finite element calculations, we introduce the following notation for the strains:

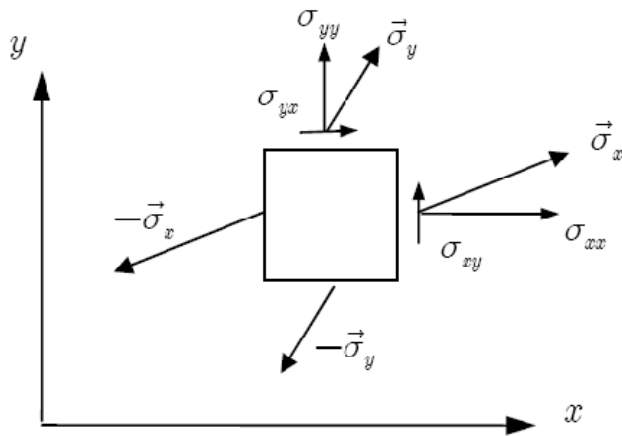
$$\boldsymbol{\varepsilon} = \begin{bmatrix} \varepsilon_x & \varepsilon_y & \gamma_{xy} \end{bmatrix}^T = \begin{bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{bmatrix} = \begin{bmatrix} \frac{\partial u_x}{\partial x} \\ \frac{\partial u_y}{\partial y} \\ \frac{\partial u_x}{\partial y} + \frac{\partial u_y}{\partial x} \end{bmatrix} = \underbrace{\begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix}}_{\nabla_s} \begin{bmatrix} u_x \\ u_y \end{bmatrix} = \nabla_s u$$

- As shown above, the symmetric gradient matrix  $\nabla_s$  is defined as:

$$\nabla_s = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix}$$

# Traction vector and stress components

- The **traction (force per unit area)** on the plane with the normal vector  $\vec{n}$  aligned along the x-axis is denoted by  $\vec{\sigma}_x$  and its vector form is  $\vec{\sigma}_x = \sigma_{xx}\vec{i} + \sigma_{xy}\vec{j}$
- Similarly, the traction with the outer normal unit vector  $\vec{n}$  aligned along the y-axis is denoted as  $\vec{\sigma}_y$  and its corresponding components are  $\vec{\sigma}_y = \sigma_{yx}\vec{i} + \sigma_{yy}\vec{j}$



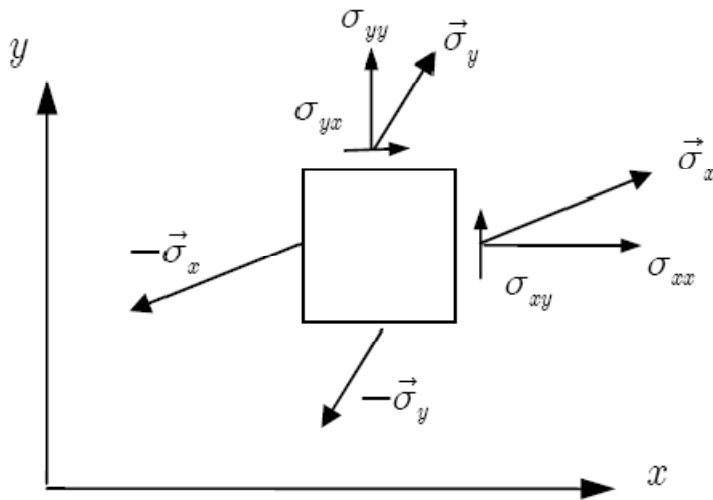
$\vec{\sigma}_x$  and  $\vec{\sigma}_y$  are called the **stress vectors** acting on the planes normal to x and y directions, respectively.

The stress state in a 2D body is described by two normal stresses  $\sigma_{xx}$  and  $\sigma_{yy}$  and shear stresses  $\sigma_{xy}$  and  $\sigma_{yx}$ .

Can you show that  $\sigma_{xy} = \sigma_{yx}$ ? (moment equilibrium)

# Stress components

- Positive stress components act in the positive direction on a positive face.
- The 1<sup>st</sup> subscript on the stress corresponds to the direction normal to the plane and the 2<sup>nd</sup> subscript denotes the direction of the force. The normal stresses are often written simply as  $\sigma_x$  and  $\sigma_y$ .



In matrix form, we denote the stress components as

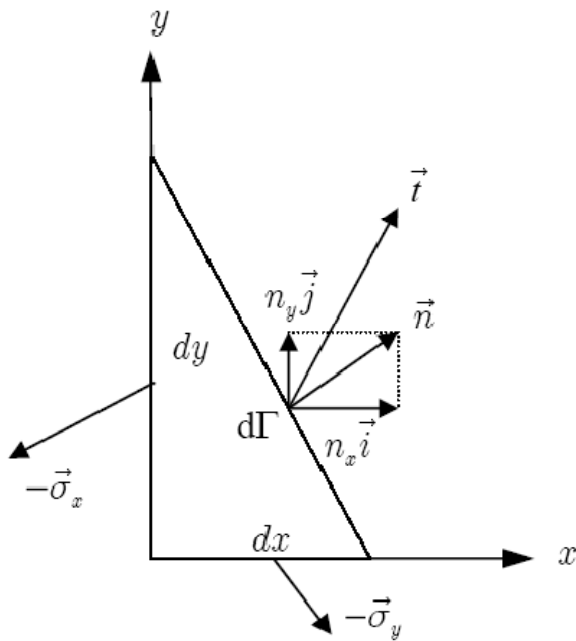
$$\sigma = \begin{bmatrix} \sigma_{xx} & \sigma_{yy} & \sigma_{xy} \end{bmatrix}^T = \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix}$$

or as

$$\tau = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} \\ \sigma_{xy} & \sigma_{yy} \end{bmatrix}$$

# Traction in an arbitrary surface with normal $\vec{n}$

- The stress vectors  $\vec{\sigma}_x$  and  $\vec{\sigma}_y$  can be used to obtain the tractions on any surface of the body with normal  $\vec{n}$ . Thus the stresses provide information about tractions on any surface at a point.



The force equilibrium of the triangular body shown requires that:

$$\vec{t}d\Gamma - \vec{\sigma}_x dy - \vec{\sigma}_y dx = 0 \Rightarrow$$

$$\vec{t}d\Gamma - \vec{\sigma}_x n_x d\Gamma - \vec{\sigma}_y n_y d\Gamma = 0 \Rightarrow$$

$$\vec{t} = \vec{\sigma}_x n_x + \vec{\sigma}_y n_y \Rightarrow$$

$$\vec{t} = (\sigma_{xx} \vec{i} + \sigma_{xy} \vec{j}) n_x + (\sigma_{yx} \vec{i} + \sigma_{yy} \vec{j}) n_y \Rightarrow$$

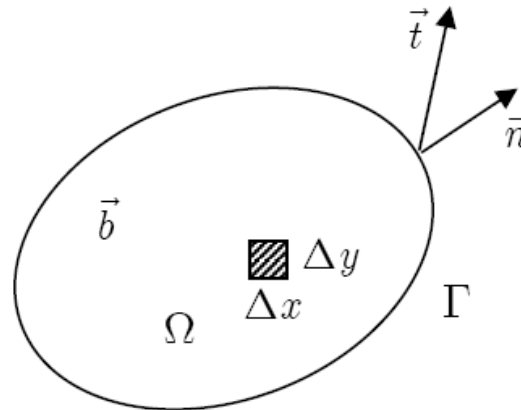
$$\vec{t} = (\underbrace{\sigma_{xx} n_x + \sigma_{yx} n_y}_{t_x}) \vec{i} + (\underbrace{\sigma_{yx} n_x + \sigma_{yy} n_y}_{t_y}) \vec{j}$$

In matrix form

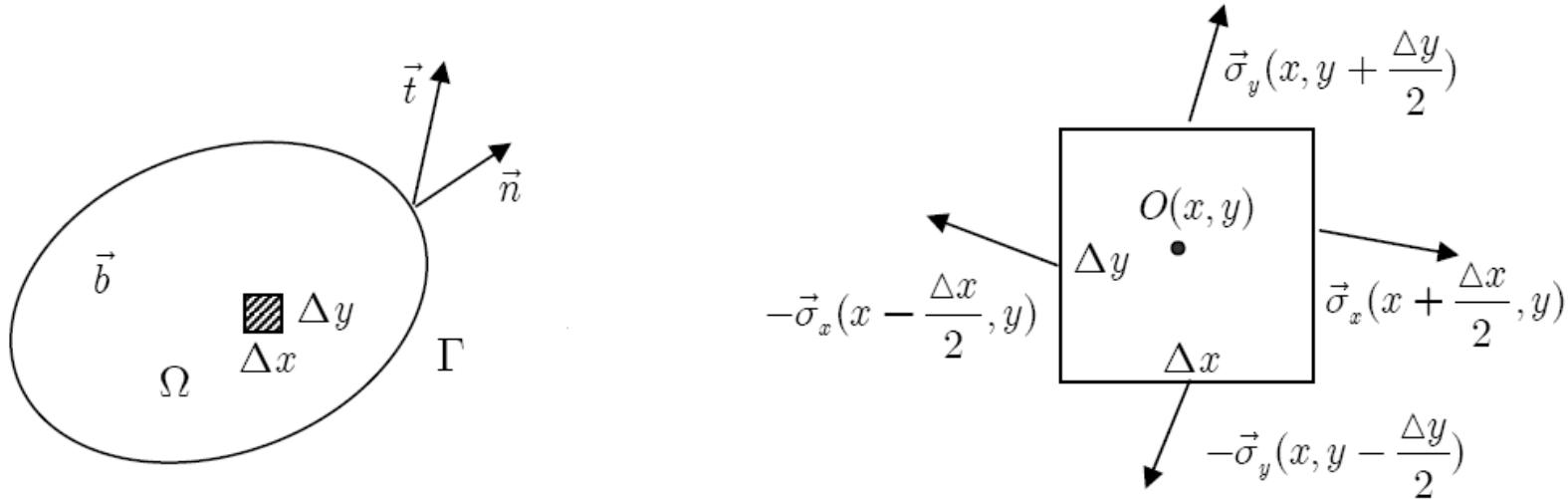
$$\underbrace{\begin{bmatrix} t_x \\ t_y \end{bmatrix}}_t = \underbrace{\begin{bmatrix} \sigma_{xx} & \sigma_{xy} \\ \sigma_{xy} & \sigma_{yy} \end{bmatrix}}_\tau \underbrace{\begin{bmatrix} n_x \\ n_y \end{bmatrix}}_n, \quad t = \tau n$$

# Stress equilibrium

- The forces acting on the body are the **traction** vector  $\vec{t} = t_x \vec{i} + t_y \vec{j}$  along the boundary  $\Gamma$  and **the body force per unit volume**  $\vec{b} = b_x \vec{i} + b_y \vec{j}$ .
- Examples of the body forces are gravity and magnetic forces, etc. Thermal stresses as we have seen can also be interpreted as body forces.



# Stress equilibrium

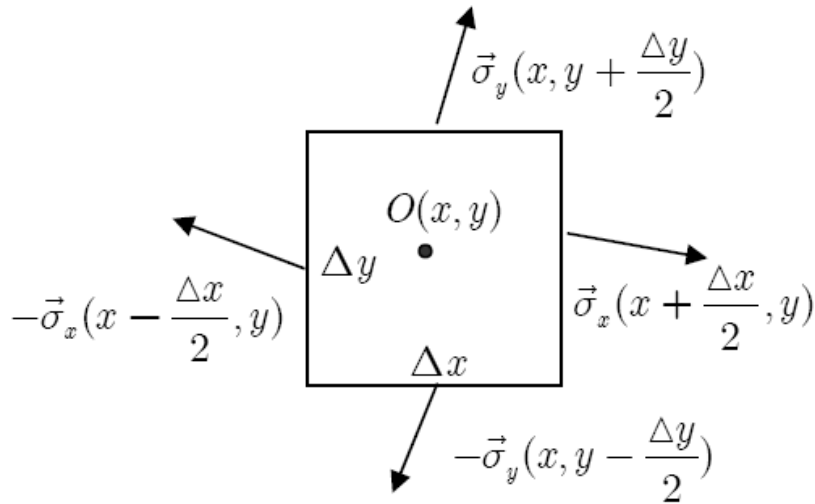


- Consider the equilibrium of the infinitesimal region (on the plane) of unit thickness:

$$-\bar{\sigma}_x\left(x - \frac{\Delta x}{2}, y\right)\Delta y + \bar{\sigma}_x\left(x + \frac{\Delta x}{2}, y\right)\Delta y - \bar{\sigma}_y\left(x, y - \frac{\Delta y}{2}\right)\Delta x + \bar{\sigma}_y\left(x, y + \frac{\Delta y}{2}\right)\Delta x + \bar{b}(x, y)\Delta x\Delta y = 0 \Rightarrow$$

$$\frac{\bar{\sigma}_x\left(x + \frac{\Delta x}{2}, y\right) - \bar{\sigma}_x\left(x - \frac{\Delta x}{2}, y\right)}{\Delta x} + \frac{\bar{\sigma}_y\left(x, y + \frac{\Delta y}{2}\right) - \bar{\sigma}_y\left(x, y - \frac{\Delta y}{2}\right)}{\Delta y} + \bar{b}(x, y) = 0 \quad \underset{\Delta x, \Delta y \rightarrow 0}{\Rightarrow}$$

# Stress equilibrium



$$\frac{\partial \bar{\sigma}_x}{\partial x} + \frac{\partial \bar{\sigma}_y}{\partial y} + \bar{b}(x, y) = 0$$

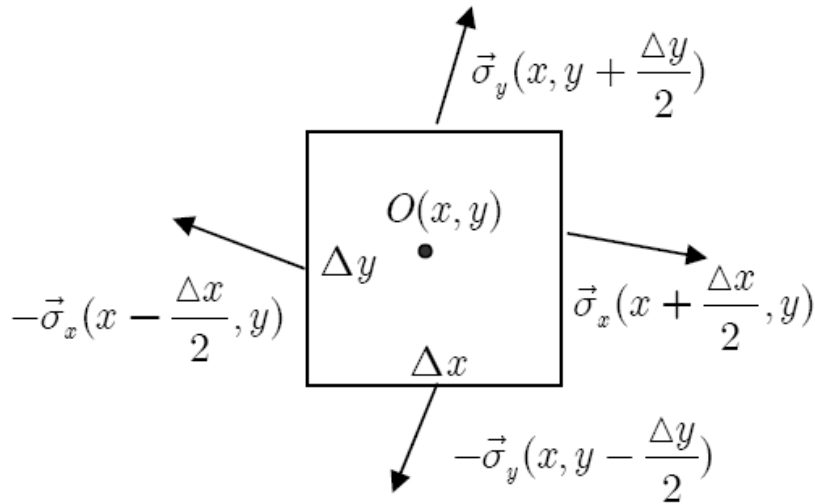
However:  $\bar{\sigma}_x = \sigma_{xx} \vec{i} + \sigma_{xy} \vec{j}$

$$\bar{\sigma}_y = \sigma_{yx} \vec{i} + \sigma_{yy} \vec{j}$$

- Combining the above two equations yields the equilibrium equations for the x- and y-directions:

$$\frac{\partial(\sigma_{xx} \vec{i} + \sigma_{xy} \vec{j})}{\partial x} + \frac{\partial(\sigma_{yx} \vec{i} + \sigma_{yy} \vec{j})}{\partial y} + \bar{b}(x, y) = 0 \Rightarrow \begin{cases} \frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} + b_x = 0 \\ \frac{\partial \sigma_{yx}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} + b_y = 0 \end{cases} \quad \text{or} \quad \begin{cases} \nabla \cdot \bar{\sigma}_x + b_x = 0 \\ \nabla \cdot \bar{\sigma}_y + b_y = 0 \end{cases}$$

# Stress equilibrium



$$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} + b_x = 0$$

$$\frac{\partial \sigma_{yx}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} + b_y = 0$$

- We can now re-write the equilibrium equation in matrix form:

$$\underbrace{\begin{bmatrix} \frac{\partial}{\partial x} & 0 & \frac{\partial}{\partial y} \\ 0 & \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix}}_{\nabla_s^T} \underbrace{\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix}}_{\sigma} + \underbrace{\begin{bmatrix} b_x \\ b_y \end{bmatrix}}_b = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \Rightarrow$$

$$\nabla_s^T \sigma + b = 0$$

# Constitutive equations

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- Constitutive equation is the relation between stress and strain. Examples include elasticity, viscoelasticity, creep, plasticity, viscoplasticity, etc.
- Here, we discuss **linear isotropic elasticity**.
- Recall that in 1D, a linear elastic material is governed by the Hooke's law  $\sigma = E\varepsilon$ , where  $E$  is Young's modulus.
- In 2D, the linear relation between the stress and strain matrices can be written as

$$\boldsymbol{\sigma} = \mathbf{D}\boldsymbol{\varepsilon}$$

where  $\mathbf{D}$  is a 3x3 matrix.

# Constitutive equations

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$$\sigma = \mathbf{D}\varepsilon$$

- This is the generalized Hooke's law.  $\mathbf{D}$  is a symmetric, positive-definite matrix.
- In 2D, the matrix  $\mathbf{D}$  depends on whether one assumes a plane stress or plane strain condition. These assumptions determine how the model is simplified from a 3D physical body to a 2D model.

# Plane strain conditions

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- A plane strain model assumes that the body is thick relative to the  $xy$ -plane in which the model is constructed.
- Consequently, the strain normal to the plane,  $\epsilon_z$  is zero and the shear strains that involve angles normal to the plane,  $\gamma_{xz}$  and  $\gamma_{yz}$  are assumed to vanish.
- When a body is thick, significant stresses can develop on the  $z$ -faces, in particular the normal stress  $\sigma_{zz}$  can be quite large.

# Plane stress

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- A plane stress model assumes that the body is thin relative to the dimensions in the  $xy$ -plane.
- In that case, we assume that no loads are applied on the  $z$ -faces of the body and that the stress normal to the  $xy$ -plane,  $\sigma_{zz}$ , vanishes.
- If a body is thin, since the stress  $\sigma_{zz}$  must vanish on the outside surfaces, there is no mechanism for developing a significant nonzero stress  $\sigma_{zz}$  within the body.

# Constitutive equations

- Here we assume an isotropic material, i.e. a material whose stress-strain law is independent of the coordinate system.
- For an isotropic material,  $\mathbf{D}$  is the same regardless of the coordinate system. Of course this is not the best approximation for all problems!

Plane stress:

$$D = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & (1-\nu)/2 \end{bmatrix}$$

Young's modulus  $E$   
and Poisson's ratio  $\nu$   
*are the only independent  
material properties for  
a linear isotropic elastic  
material*

Plane strain:

$$D = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & 0 \\ \nu & 1-\nu & 0 \\ 0 & 0 & (1-2\nu)/2 \end{bmatrix}$$

# Constitutive equations

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- For an isotropic elastic material we have only 2 independent material constants:  $E$  and  $\nu$  (of course  $\mathbf{D}$  can be written using any two other elastic material constants such as the shear modulus  $G=E/2(1+\nu)$  and the bulk modulus  $K=E/3(1-\nu)$ ).
- Note that for plane strain, as  $\nu \rightarrow 0.5$ ,  $\mathbf{D}$  becomes infinite.
- A Poisson's ratio of 0.5 corresponds to an incompressible material.
- Modeling incompressible materials for plane strain (and 3D) problems requires special attention in finite element analysis (and special elements).

# Strong form of an elasticity problem

- Equilibrium equation:  $\nabla_s^T \boldsymbol{\sigma} + \mathbf{b} = 0$  or equivalently:  $\bar{\nabla} \cdot \bar{\boldsymbol{\sigma}}_x + b_x = 0$   
 $\bar{\nabla} \cdot \bar{\boldsymbol{\sigma}}_y + b_y = 0$

- Kinematics equation (strain-displacement relation):

$$\boldsymbol{\varepsilon} = \nabla_s u$$

- Constitutive equation (stress-strain relation):

$$\boldsymbol{\sigma} = \mathbf{D}\boldsymbol{\varepsilon}$$

- Boundary conditions: The portion of the boundary where the traction is prescribed is denoted by  $\Gamma_t$  and the portion of the boundary where the displacement is prescribed  $\Gamma_u$ .

- ✓ The traction boundary condition is written as  $\boldsymbol{\tau} n = \bar{\mathbf{t}}$  on  $\Gamma_t$  or equivalently:

$$\bar{\boldsymbol{\sigma}}_x \cdot \bar{\mathbf{n}} = \bar{t}_x, \bar{\boldsymbol{\sigma}}_y \cdot \bar{\mathbf{n}} = \bar{t}_y \text{ on } \Gamma_t$$

- ✓ The displacement boundary condition is written as  $\bar{\mathbf{u}} = \bar{\bar{\mathbf{u}}}$  on  $\Gamma_u$

# Strong form

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- **Essential boundary condition:** The displacement boundary condition is the essential boundary condition satisfied by the displacement field.
- **Natural boundary condition:** The traction boundary condition is a natural boundary condition.
- **The displacement and traction cannot both be prescribed on the same part of the boundary, thus**

$$\Gamma_u \cap \Gamma_t = \emptyset.$$

- **However, on any portion of the boundary, either the displacement or the traction must be prescribed, so**

$$\Gamma_u \cup \Gamma_t = \Gamma$$

# Strong form for isotropic linear elasticity

- Find the displacement field  $\bar{u}$  on  $\Omega$  such that:

$$\bar{\nabla} \cdot \bar{\sigma}_x + b_x = 0, \bar{\nabla} \cdot \bar{\sigma}_y + b_y = 0 \quad \text{on } \Omega$$

$$\text{where } \sigma = D \nabla_s u$$

with

$$\bar{\sigma}_x \cdot \bar{n} = \bar{t}_x, \bar{\sigma}_y \cdot \bar{n} = \bar{t}_y \quad \text{on } \Gamma_t$$

$$\bar{u} = \bar{u} \quad \text{on } \Gamma_u$$

# Weak form for isotropic linear elasticity

- We pre-multiply the equilibrium equations in  $x$  and  $y$  directions and the two natural boundary conditions by the corresponding weight functions and integrate over the corresponding domains as follows:

$$\bar{\nabla} \cdot \bar{\sigma}_x + b_x = 0, \bar{\nabla} \cdot \bar{\sigma}_y + b_y = 0 \text{ on } \Omega \Rightarrow$$

$$\int_{\Omega} (\bar{\nabla} \cdot \bar{\sigma}_x + b_x) w_x d\Omega = 0 \quad \forall w_x \in U_0,$$

$$\int_{\Omega} (\bar{\nabla} \cdot \bar{\sigma}_y + b_y) w_y d\Omega = 0 \quad \forall w_y \in U_0.$$

$$\bar{\sigma}_x \cdot \bar{n} = \bar{t}_x, \bar{\sigma}_y \cdot \bar{n} = \bar{t}_y \text{ on } \Gamma_t \Rightarrow \int_{\Gamma_t} w_x (\bar{\sigma}_x \cdot \bar{n} - \bar{t}_x) d\Gamma = 0 \quad \forall w_x \in U_0,$$

$$\int_{\Gamma_t} w_y (\bar{\sigma}_y \cdot \bar{n} - \bar{t}_y) d\Gamma = 0 \quad \forall w_y \in U_0.$$

# Weak form for isotropic linear elasticity

$$\int_{\Omega} (\bar{\nabla} \cdot \bar{\sigma}_x + b_x) w_x d\Omega = 0 \quad \forall w_x \in U_0,$$

$$\int_{\Omega} (\bar{\nabla} \cdot \bar{\sigma}_y + b_y) w_y d\Omega = 0 \quad \forall w_y \in U_0.$$

- Apply Green's theorem (integration by parts) to the first term in each of these equations and account for  $w_x = w_y = 0$  on  $\Gamma_u$

$$\int_{\Gamma_t} w_x \bar{\sigma}_x \cdot \bar{n} d\Gamma - \int_{\Omega} \bar{\nabla} w_x \cdot \bar{\sigma}_x d\Omega + \int_{\Omega} b_x w_x d\Omega = 0 \quad \forall w_x \in U_0,$$

$$\int_{\Gamma_t} w_y \bar{\sigma}_y \cdot \bar{n} d\Gamma - \int_{\Omega} \bar{\nabla} w_y \cdot \bar{\sigma}_y d\Omega + \int_{\Omega} b_y w_y d\Omega = 0 \quad \forall w_y \in U_0.$$

- Adding the two equations gives:

$$\int_{\Omega} (\bar{\nabla} w_x \cdot \bar{\sigma}_x + \bar{\nabla} w_y \cdot \bar{\sigma}_y) d\Omega = \int_{\Gamma_t} \bar{w} \cdot \bar{t} d\Gamma + \int_{\Omega} \bar{w} \cdot \bar{b} d\Omega \quad \forall w_x \in U_0$$

# Weak form for isotropic linear elasticity

$$\int_{\Omega} \left( \bar{\nabla} w_x \cdot \bar{\sigma}_x + \bar{\nabla} w_y \cdot \bar{\sigma}_y \right) d\Omega = \int_{\Gamma_t} \bar{w} \cdot \bar{t} d\Gamma + \int_{\Omega} \bar{w} \cdot \bar{b} d\Omega \quad \forall w_x \in U_0$$

- The left hand side can be simplified by noticing that:

$$\begin{aligned} \bar{\nabla} w_x \cdot \bar{\sigma}_x + \bar{\nabla} w_y \cdot \bar{\sigma}_y &= \frac{\partial w_x}{\partial x} \sigma_{xx} + \frac{\partial w_x}{\partial y} \sigma_{xy} + \frac{\partial w_y}{\partial x} \sigma_{xy} + \frac{\partial w_y}{\partial y} \sigma_{yy} = \\ &= \begin{bmatrix} \frac{\partial w_x}{\partial x} & \frac{\partial w_y}{\partial y} & \frac{\partial w_x}{\partial y} + \frac{\partial w_y}{\partial x} \end{bmatrix} \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix} = \left( \underbrace{\begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial y} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} \end{bmatrix}}_{\nabla_s} \begin{bmatrix} w_x \\ w_y \end{bmatrix} \right)^T \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{bmatrix} = \left( \nabla_s w \right)^T \sigma \end{aligned}$$

# Weak form for isotropic linear elasticity

$$\int_{\Omega} \left( \bar{\nabla} w_x \cdot \bar{\sigma}_x + \bar{\nabla} w_y \cdot \bar{\sigma}_y \right) d\Omega = \int_{\Gamma_t} \bar{w} \cdot \bar{t} d\Gamma + \int_{\Omega} \bar{w} \cdot \bar{b} d\Omega \quad \forall w_x \in U_0$$

$$\bar{\nabla} w_x \cdot \bar{\sigma}_x + \bar{\nabla} w_y \cdot \bar{\sigma}_y = \left( \nabla_S w \right)^T \sigma$$

- The matrix form of the weak form becomes:

$$\int_{\Omega} \left( \nabla_S w \right)^T \sigma d\Omega = \int_{\Gamma_t} \bar{w} \cdot \bar{t} d\Gamma + \int_{\Omega} \bar{w} \cdot \bar{b} d\Omega \quad \forall w_x \in U_0$$

# Weak form for isotropic linear elasticity

- Substituting the kinematic  $\varepsilon = \nabla_s u$  and constitutive  $\sigma = \mathbf{D}\varepsilon$ , relations, the resulting weak form in 2D can be summarized as:

Find  $u \in U$  on  $\bar{\Omega}$  such that :

$$\int_{\Omega} \left( \nabla_s w \right)^T \sigma d\Omega = \int_{\Gamma_t} \bar{w} \cdot \vec{t} d\Gamma + \int_{\Omega} \bar{w} \cdot \vec{b} d\Omega \quad \forall w \in U_0$$

where  $U = \left\{ u : u \in H^1, u = \bar{u} \text{ on } \Gamma_u \right\}$  and

$$U_0 = \left\{ w : w \in H^1, w = 0 \text{ on } \Gamma_u \right\}$$

**Can you think a virtual work interpretation of the above equation?**